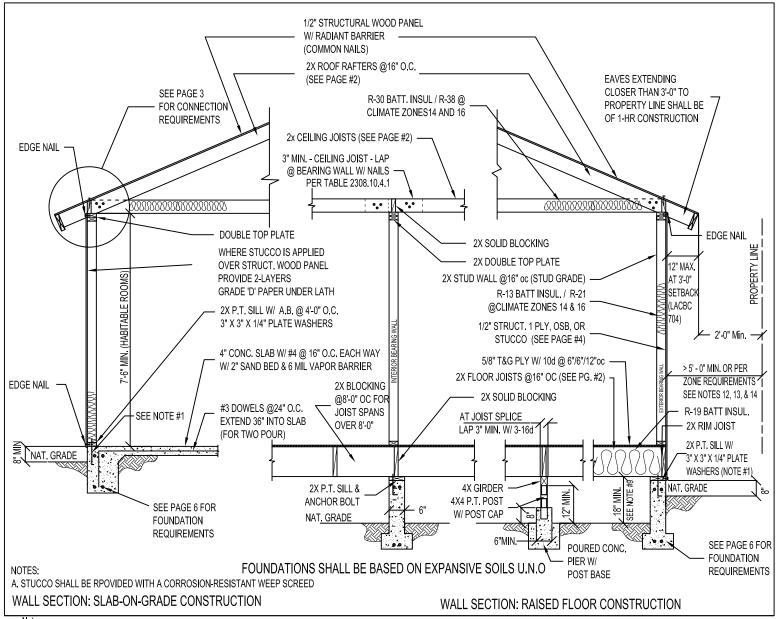


2008 LOS ANGELES COUNTY BUILDING CODE

TYPE V CONSTRUCTION-WOOD FRAME RESIDENTIAL BUILDINGS DEAD LOAD SHALL NOT EXCEED 15 PSF FOR COMBINED ROOF AND CEILING. OR EXTERIOR WALLS, OR FLOORS, AND PARTITIONS, (LACBC, CHAPTER 23, SECTION 2308)

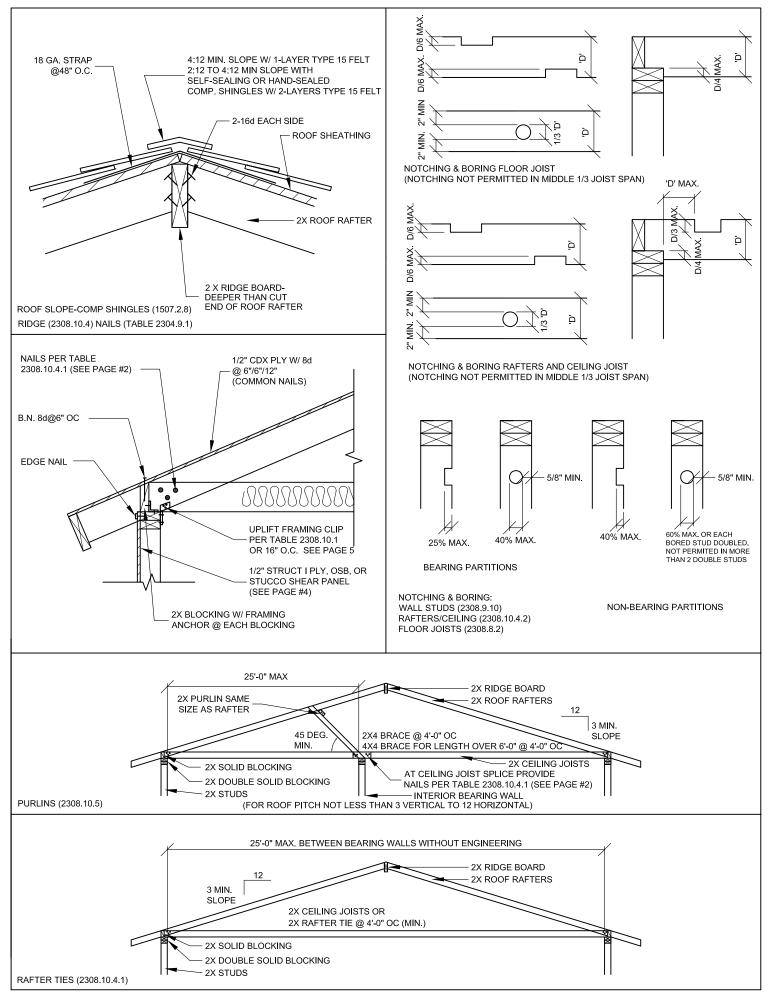
TYPE V CONSTRUCTION OF BUILDINGS BY CONSTRUCTION MATERIALS AND METHODS. IT IS THE LEAST RESTRICTIVE PERMITTED BY LA COUNTY BUILDING CODE AND INCLUDES LIGHT, WOOD-FRAME CONSTRUCTION, THIS SHEET IS FOR INFORMAION AND REFERENCE ONLY AND IS NOT A SUBSTITUTE FOR ACCURATE DRAWINGS PREPARED FOR EACH PROPOSED CONSTRUCTION PROJECT.



Notes:

- Anchor bolts: 5/8" Ø x 10"; minimum 7" embedment, with minimum 2 anchor bolts per piece, located not more than 12" or less than 4" from each end of the piece. 1.
- All foundation plates or sills and sleepers on a concrete or masonry slab, which is in direct contact with earth, and sills that rest on concrete or masonry foundations, shall be pressure treated wood. 2.
- 3. Minimum concrete Strength: 2500psi
- Bearing walls and braced wall panels require continuous footings. 4.
- Soil report is required if the proposed construction is located in a liquefaction, landslide, Alquist-Priolo, Sierra Madre or other earthquake fault zone. 5.
- 6. Where interior walls are shear wall panels, wall framing and sheathing shall extend to the roof sheathing.
- Under floor areas shall be ventilated by approved mechanical means or by openings into the under-floor area walls. Such openings shall have a net area of not less than 1 sq. ft. for each 150 sq. feet of under-floor 7. area, Openings shall be located as close as possible to corners and provide cross ventilation, the openings shall be approximately equally distributed along the length of at least two sides, Corrosion resistant mesh w/ 1/4" openings.
- The net free ventilating area shall not be less than 1/150 of the space ventilated, with 50% of the required ventilating area provided by ventilators located in the upper portion of the space to be ventilated at least 3 foot above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.
- For stem walls greater than 24" high: Refer to local jurisdiction requirements. 9.
- For Very High Fire Hazard Severity Zone (VHFHSZ): Additional requirements apply, see VHFHSZ correction sheet.
- 11. Provide a minimum of 1" airspace between insulation and the roof sheathing.
- Exterior walls of dwellings, guesthouses, garages, carports and/or accessory structures closer than 5ft. to the property line shall be 1-hour fire-resistance-rated construction. 12
- No openings shall be permitted in the exterior walls, including vents, of Group R-3 and Group U Occupancies where the exterior wall is 3-ft, or closer to the property line. 13.
- Where the exterior wall of Group R-3 is located > 3-ft, and ≤ 5-ft, to the property line, the area of protected and unprotected openings is limited to 25% of the wall area, including vents. 14.
- 15. Footings on or adjacent to slopes shall meet the requirements of section 1805.3

| | d (up to 10 psf) Live L Load: 6 psf (TABLE | • | ALL | OWABLE SPAN | S FOR DF #2 CE (TABLE 2308. | | TS (DF-LA | RCH) | | | |
|--|--|---|--|--|---|--|--|--|-------------------------|-------------------------|--|
| | . , | ` " | | | ALLOWABL | ALLC | WABL | E SPA | N | | |
| RAFTER SIZE | SPACING | ALLOWABLE SPAN | JOIST SIZE | SPACING | Dead Load Live Load | Dead Load 10 psf / Live Load 20 psf | | | | | |
| | 24" | 11'-9" | | 24" | 9'-10 | 9'-10" | | 7'-2" | | | |
| 2X6 | 16" | 14'-1" | 2x4 | 16" | 11'-3" | | 8'-9" | | | | |
| | 12" | 15'-6" | | 12" | 12'-5 | " | 9'-10" | | | | |
| | 24" | 14'-10" | | 24" | 14'-10 |)" | 10'-6" | | | | |
| 2X8 | 16" | 18'-2" | 2X6 | 16" | 17'-8 | | 12'-10" | | | | |
| | 12" | 20'-5" | | 12" | 19'-6 | | | 14'-10 | 4'-10" | | |
| | 24" | 18'-2" | | 24" | 18'-9 | 9" 13'-3 | | | | | |
| 2X10 | 16" | 22'-3" | 2X8 | 16" | 23'-0 | | | 16'-3 | • | | |
| | 12" | 25'-8" | | 12" | 25'-8 | " | 18'-9" | | • | | |
| | 24" | 21'-0" | | 24" | 22'-11 | | | | • | | |
| 2X12 | 16" | 25'-9" | 2X10 | 16" | 26'-0 | " | 19-10" | | | | |
| | 12" | 26'-0" | 12" | | 26'-0 | " | | 22'-11 | " | | |
| ALLOWARIE SDAI | NS FOR DF #2 FLOO | | | | | | ER TIE CO | | | r | |
| ARCH) | 13 FOR DF #2 FLOO | K 301313 (DI- | ALLOWABLE | SPANS FOR D | F#1 FLOOR | | | | | | |
| .ARCH) | (TABLE 2308.8(2)) | | GIRI | DERS (DF-LARC | CH) | (TABLE 23 Required numb | per of 16d com | mon nail: | s per con | nection | |
| Dead Load (up to 1 | 0 psf) | | Max. Floor Dead | l Load: 15 psf | wood members | | shall be sufficient size to prevent splittin g. Split members shall be removed and replaced. | | | | |
| Live Load: 40 psf | | | SPA | N | GIRDER SIZE | D4 | TIE ROOF SPAN(FT. | | | l(FT.) | |
| JOIST SIZE | SPACING | ALLOWABLE SPAN | PARTITIONS | NO PARTITIONS | | RAFTER SLOPE | SPACIN G | 12 | 20 | 25 | |
| | | | 5'-3" | 5'-8" | 4x6 | | 12 | 4 | 6 | 8 | |
| | 24" | 8'-1" | 6'-10" | 7'-4" | 4x8 | | 16 | 5 | 7 | 10 | |
| 2X6 | 16" | 9'-9" | | | | 3:12 | 24 | 7 | 11 | 15 | |
| | 12" | 10'-9" | HEADER SPANS | FOR DF EXTERI | OR BEARING | | 32 | 10 | 14 | 19 | |
| | 24" | 10'-3" | WALLS BASI | ED ON 28' BUILDI | NG WIDTH | | 48 | 14 | 21 | 29 | |
| 2X8 | 16" | 12'-7" | | TABLE 2308.9.5) | | | 12 | 3 | 4 | 5 | |
| 2/10 | 12" | 14'-2" | SPAN | BEAM SIZE | | | 16 | 3 | 5 | 7 | |
| | 24" | 12'-7" | | DEAM OILE | | 4:12 | 24 | 4 | 7 | 10 | |
| 2X10 | 16" | 15'-5" | 4'-8" | 4x6 | | 7.12 | 32 | 6 | 9 | 13 | |
| 2710 | 12" | 17'-9" | | | | | 48 | 8 | 14 | 19 | |
| | 24" | 14'-7" | 5'-11" | 4x8 | | | 12 | 3 | 3 | 4 | |
| 2X12 | 16" | 17'-10" | | | | | | | | | |
| | | | | | | | | | | | |
| 2712 | | | 7'-3" | 4x10 | | 5.12 | 16 | 3 | 4 | 5 | |
| 2812 | 12" | 20'-7" | | | | 5:12 | 24 | 4 | 6 | 8 | |
| 2/1/2 | | | 7'-3" 8'-5" | 4x10 4x12 | | 5:12 | | | | | |
| | 12" S SPECIFIED IN THE TABLE SH | 20'-7" ALL BE PROVIDED AT EACH C | 8'-5" | 4x12 | | IN BRACING ARE | 24 32 48 PROVIDED, RA | 4 5 7 | 6 8 11 | 8 10 15 | |
| *THE NUMBER OF NAILS | 12" S SPECIFIED IN THE TABLE SH REDUCED PROF | 20'-7" | 8'-5" ONNECTION. WHEN FULL N IN RAFTER SPAN; NO LI HEATHING | 4x12 | ALL BE PROVIDED AT E | IN BRACING ARE | 24 32 48 PROVIDED, RAN. | 4 5 7 AFTER TIE | 6 8 11 NAILING | 8 10 15 | |
| *THE NUMBER OF NAILS ALLOWABLE SPAI CONTINUOUS OVE | 12" S SPECIFIED IN THE TABLE SH REDUCED PROF | 20'-7" ALL BE PROVIDED AT EACH C PORTIONAL TO THE REDUCTIO OR OSB AND ROOF S | 8'-5" ONNECTION. WHEN FULL N IN RAFTER SPAN; NO LI HEATHING | 4x12 | ALL BE PROVIDED AT E | IN BRACING ARE | 24 32 48 PROVIDED, RAN. | 4 5 7 AFTER TIE | 6 8 11 NAILING | 8 10 15 | |
| *THE NUMBER OF NAILS ALLOWABLE SPAI CONTINUOUS OVE | 12" S SPECIFIED IN THE TABLE SH REDUCED PROP NS FOR PLYWOOD C | 20'-7" ALL BE PROVIDED AT EACH C PORTIONAL TO THE REDUCTIO OR OSB AND ROOF S | 8'-5" ONNECTION, WHEN FULL N IN RAFTER SPAN; NO LI HEATHING AR TO ROOF | 4x12 HEIGHT INTERIOR BEA | ALL BE PROVIDED AT E | IN BRACING ARE | 24 32 48 PROVIDED, RANN. | 4 5 7 AFTER TIE | 6 8 11 NAILING | 8 10 15 | |
| *THE NUMBER OF NAILS ALLOWABLE SPAI CONTINUOUS OVE SHEA SPAN RATING FLOOR/ ROOF SPAN | 12" SPECIFIED IN THE TABLE SH REDUCED PROP NS FOR PLYWOOD C R TWO OR MORE SI THING SPAN THICKNESS | ALL BE PROVIDED AT EACH C PORTIONAL TO THE REDUCTIO OR OSB AND ROOF S PANS-PERPENDICUL MAX. SPAN (IN.) EDGE SUPPORT(2X BLOCKING) | 8'-5" ONNECTION. WHEN FULL. N IN RAFTER SPAN; NO LI HEATHING AR TO ROOF NO EDGE SUPPORT FOR 1/2", MAX. SPAN 24" | 4x12 HEIGHT INTERIOR BEAESS THAN 3 NAILS SHA | NOTE: AF S(PSF) LIVE LOADS | IN BRACING AREACH CONNECTIO | 24 32 48 PROVIDED, RAN. PANELS 24 FLOG MAX. SPA | 4 5 7 AFTER TIE UP OR V DR AN(IN.) | 6 8 11 NAILING | 8 10 15 MAY BI | |
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| *THE NUMBER OF NAILS ALLOWABLE SPAI CONTINUOUS OVE SHEA SPAN RATING FLOOR/ ROOF SPAN 24/0 24/16 | 12" S SPECIFIED IN THE TABLE SH REDUCED PROF NS FOR PLYWOOD C R TWO OR MORE SI THING SPAN THICKNESS 7/16, 1/2 7/16, 1/2 | ALL BE PROVIDED AT EACH C PORTIONAL TO THE REDUCTIO OR OSB AND ROOF S PANS-PERPENDICUL MAX. SPAN (IN.) EDGE SUPPORT(2X BLOCKING) 24 24 | 8'-5" ONNECTION. WHEN FULL N IN RAFTER SPAN; NO LI HEATHING AR TO ROOF NO EDGE SUPPORT FOR 1/2", MAX. SPAN 24" 20 24 | 4x12 HEIGHT INTERIOR BEASS THAN 3 NAILS SHA LOAD TOTAL LOADS 40 50 | NOTE: AF S(PSF) LIVE LOADS 30 40 | IN BRACING AREACH CONNECTIO | 24 32 48 PROVIDED, R.N. PANELS 24 FLOC MAX. SPA ges with to nts or with | 4 5 7 AFTER TIE UP OR V DR AN(IN.) | 6 8 11 NAILING | 8 10 15 MAY BI | |
| *THE NUMBER OF NAILS ALLOWABLE SPAI CONTINUOUS OVE SHEA SPAN RATING FLOOR/ ROOF SPAN 24/0 24/16 32/16 | 12" S SPECIFIED IN THE TABLE SH REDUCED PROF NS FOR PLYWOOD C R TWO OR MORE SI THING SPAN THICKNESS 7/16, 1/2 7/16, 1/2 15/32, 1/2, 5/8 | ALL BE PROVIDED AT EACH CONTIONAL TO THE REDUCTION OF SPANS-PERPENDICUL MAX. SPAN (IN.) EDGE SUPPORT(2X BLOCKING) 24 24 32 | 8'-5" ONNECTION. WHEN FULL. N IN RAFTER SPAN; NO LI HEATHING AR TO ROOF NO EDGE SUPPORT FOR 1/2", MAX. SPAN 24" 20 24 28 | 4x12 HEIGHT INTERIOR BEA ESS THAN 3 NAILS SHA LOAD TOTAL LOADS 40 50 40 | NOTE: AF S(PSF) LIVE LOADS 30 40 30 | IN BRACING AREACH CONNECTIO | 24 32 48 PROVIDED, R.N. PANELS 24 FLOG MAX. SPA ges with to nts or with | 4 5 7 AFTER TIE UP OR V DR AN(IN.) | 6 8 11 NAILING | 8 10 15 MAY B | |
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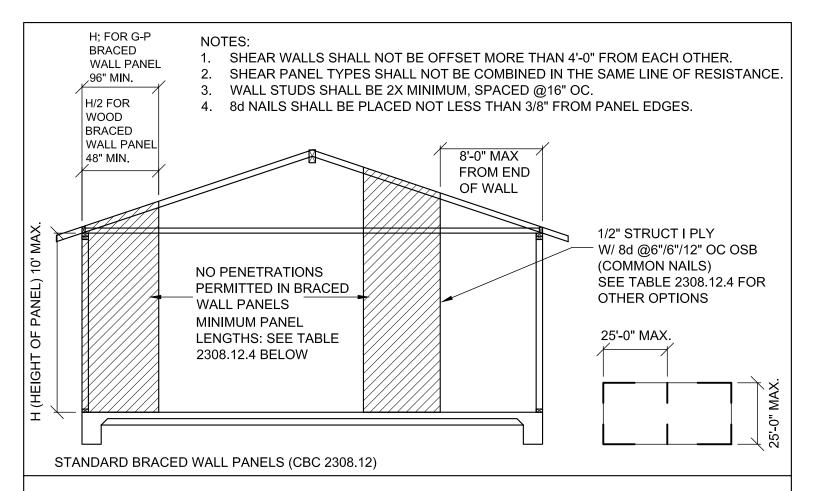


TABLE 2308.12.4

WALL BRACING IN SEISMIC DESIGN CATEGORIES D AND E (Minimum Length of Wall Bracing per each 25 Linear Feet of Braced Wall Line $^{\rm a,\,e}$)

| CONDITION | SHEATHING TYPE | S _{DS} < 0.50 | 0.50 < S _{DS} < 0.75 | 0.75 < S _{DS} < 1.00 | S _{DS} > 1.00 |
|-----------|-------------------|------------------------|-------------------------------|-------------------------------|------------------------|
| 0 | G-P° | 10 feet 8 inches | 14 feet 8 inches | 18 feet 8 inches | 25 feet 0 inches |
| One Story | S-W ^d | 5 feet 4 inches | 8 feet 0 inches | 9 feet 4 inches | 12 feet 0 inches |

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

- a. Minimum length of panel bracing of one face of the wall for S-W sheathing shall be at least 4'-0" long or both faces of the wall for G-P sheathing shall be at least 8'-0" long; h/w ratio shall not exceed 2:1. For S-W panel bracing of the same material on two faces of the wall, the minimum length is permitted to be one-half the tabulated value but the h/w ratio shall not exceed 2:1 and design for uplift is required.
- b. G-P = gypsum board, and *portland cement* plaster or gypsum sheathing boards; S-W = wood structural panels.
- c. Nailing as specified below shall occur at all panel edges at studs, at top and bottom plates and, where occurring, at blocking:

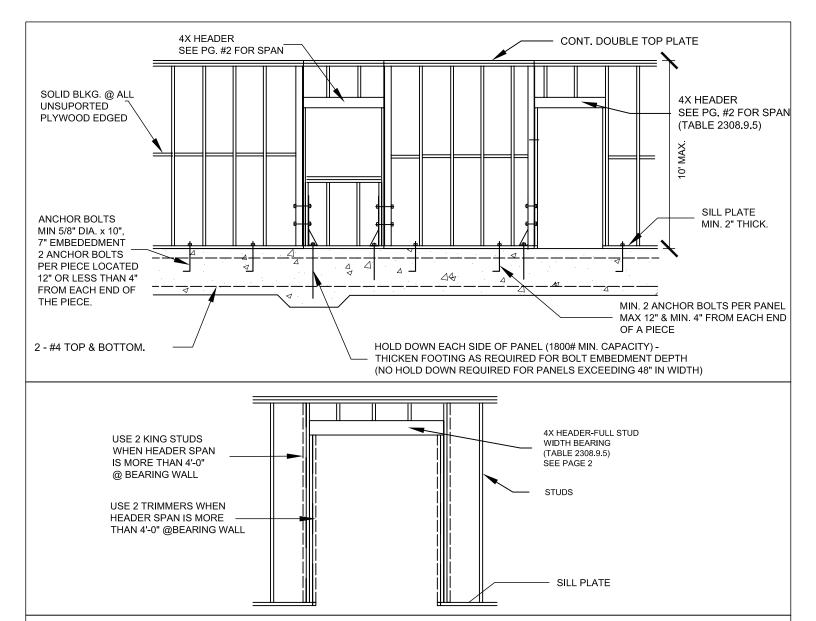
For 1/2-inch gypsum board, 5d (0.113 inch diameter) cooler nails at 7 inches on center;

For 5/8-inch gypsum board, No 11 gage (0.120 inch diameter) cooler nails at 7 inches on center;

For gypsum sheathing board, 1-3/4 inches long by 7/16-inch head, diamond point galvanized nails at 4 inches on center:

For gypsum lath, No. 13 gage (0.092 inch) by 1-1/8 inches long, 19/64-inch head, plasterboard at 5 inches on center; For Portland cement plaster, No. 11 gage (0.120 inch) by 11/2 inches long, 7/16- inch head at 6 inches on center;

- d. S-W sheathing shall be 15/32" thick nailed with 8d nails, at 6:6:12.
- e. S_{DS} > 1.00 shall apply to all projects, unless the design spectral response acceleration, S_{DS} is provided to show otherwise.



| REQUIRED RATING OF APPROVED UPLIFT CONNECTORS (pounds) ^{a,b,c,e,f,g,h} (TABLE 2308.10.1) | | | | | | | | | | |
|---|----------------------------------|------|------|------|------|------|------|----------------------------|--|--|
| BASIC WIND SPEED | ASIC WIND SPEED ROOF SPAN (feet) | | | | | | | | | |
| (3 second gust) | 12 | 20 | 24 | 28 | 32 | 36 | 40 | (pounds/feet) ^d | | |
| 85 | -72 | -120 | -145 | -169 | -193 | -217 | -241 | -38.55 | | |
| 90 | -91 | -151 | -181 | -212 | -242 | -272 | -302 | -43.22 | | |
| 100 | -131 | -281 | -262 | -305 | -349 | -393 | -436 | -53.36 | | |
| 110 | -175 | -292 | -351 | -409 | -467 | -526 | -584 | -64.56 | | |

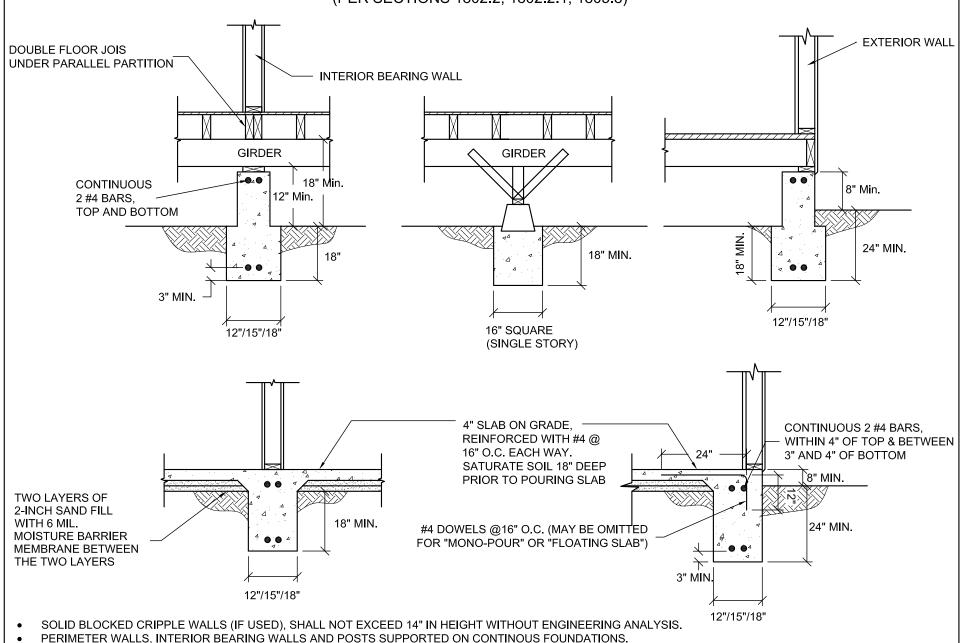
| | MEAN ROOF HEIGHT (feet) | | | | | | | | | |
|----------|-------------------------|------|------|------|------|------|------|------|------|------|
| EXPOSURE | 15 | 20 | 25 | 30 | 35 | 40 | 45 | 50 | 55 | 60 |
| В | 1.00 | 1.00 | 1.00 | 1.00 | 1.05 | 1.09 | 1.12 | 1.16 | 1.19 | 1.22 |
| С | 1.21 | 1.29 | 1.35 | 1.40 | 1.45 | 1.49 | 1.53 | 1.56 | 1.59 | 1.62 |
| D | 1.47 | 1.55 | 1.61 | 1.66 | 1.70 | 1.74 | 1.78 | 1.81 | 1.84 | 1.87 |

- The uplift connection requirements are based on a 30-foot mean roof height located in Exposure. For Exposure C or D and for other mean roof heights, multiply the loads in Table 1 by the adjustment coefficients in Table 2.
- The uplift connection requirements are based on the framing being spaced 24 inches on center. Multiply by 0.67 for framing spaced 16 inches on center and multiply by 0.5 for framing spaced 12 inches
- The uplifft connection requirements include an allowance for 10 pounds of dead load.
- The uplift connection requirements do not account for the effects of everhands. The magnitude of the above loads shall be increased by adding the overhang loads found in the table. The overhang loads are also based on framing spaced 24 inches on center. The overhang loads given shall be multiplied by the overhand projection added to the roof uplift value in the table.
- The uplift connection requirements are based upon wind loading on end zones as defined in Figure 6-2 of ASCE 7. Connection loads for connections located a distance of 20 percent of the least horizontal dimension of the building from the corner of the building are permitted to be reduced by multiplying the table connection value by 0.7 and multiplying the overhang load by 0.8. For wall-to-wall and wall-to-foundation connections, the capacity of the uplift connector is permitted to be reduced by 100 pounds for each full wall above. (For example, if a 500 pound rated connector is
- used on the roof framing, a 400-pound rated connector is permitted at the next floor level down.)
- Interpolation is permitted for intermediate values of basic wind speeds and roof spans.
- The rated capacity of approved tie-down devices is permitted to include up to a 60-percent increase for wind effects where allowed by material specifications.



FOUNDATION SYSTEM ON EXPANSIVE SOIL FOR 1 OR 2 STORY R-3/ ACCESSORY U OCCUPANCIES

(PER SECTIONS 1802.2, 1802.2.1, 1805.8)



- 12"/15"/18" MIN. FOOTING FOR SUPPORTING ONE FLOOR. TWO FLOORS. AND THREE FLOORS RESPECTIVELY.
- SHEAR TRANSFER DETAILS AND OTHER REQUIREMENTS NOT SHOWN FOR CLARITY.

